

CE-084 Wastewater Treatment II - MBBR

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Biological Wastewater Treatment II – MBBR Processes

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COURSE CONTENT

1. Introduction

Biological wastewater treatment is very widely used for removal of biodegradable materials from wastewater. The first course in this sequence, Biological Wastewater Treatment I – Activated Sludge, starts with a discussion of the biochemical oxygen demand that is created by biodegradable materials in water and the reason why such materials must be removed from wastewater. This course is about the Moving Bed Biofilm Reactor (MBBR) wastewater treatment process, including background information about the process and a description of the process, as well as process design calculations for several different configurations of the MBBR process and numerous example calculations.

2. Learning Objectives

At the conclusion of this course, the student will

- Know the differences between attached growth and suspended growth biological wastewater treatment processes
- Be familiar with the components and general configuration of an MBBR wastewater treatment process
- Be able to calculate the loading rate of a wastewater constituent to an MBBR process (in lb/day and g/day) for a specified wastewater flow rate and constituent concentration
- Be able to calculate the required carrier surface area for an MBBR wastewater treatment process for a specified SALR and loading rate
- Be able to calculate the required MBBR tank volume for specified carrier surface area, carrier specific surface area, and the carrier fill %.

- Be able to calculate the liquid volume in an MBBR tank for known tank volume, carrier volume and carrier % void space
- Be able to calculate the BOD, NH₃-N, or NO₃-N removal rate for known values of the surface area removal rate (SARR) and design carrier surface area
- Be able to calculate an estimated effluent BOD, NH₃-N, or NO₃-N concentration based on known values of the appropriate loading rate, estimated removal rate, and design wastewater flow rate
- Be able to make process design calculations for a post-Anoxic denitrification
 MBBR process, including required tank sizes, estimated effluent concentrations,
 alkalinity requirement and carbon source requirement
- Be able to make process design calculations for a pre-anoxic denitrification
 MBBR process, including required tank sizes, estimated effluent concentrations, and alkalinity requirements

3. Topics Covered in this Course

- I. Description of the MBBR (Moving Bed Biofilm Reactor) Process
- II. Single Stage BOD Removal MBBR Process Design Calculations
- III. Two-Stage BOD Removal MBBR Process Design Calculations
- IV. Single Stage Nitrification MBBR Process Design Calculations
- V. Two-Stage BOD Removal and Nitrification MBBR Process Design Calculations
- VI. Denitrification Background Information
- VII. Post-Anoxic Denitrification MBBR Process Design Calculations
- VIII. Pre-Anoxic Denitrification MBBR Process Design Calculations

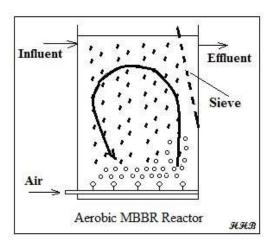
4. Description of the MBBR (Moving Bed Biofilm Reactor) Process

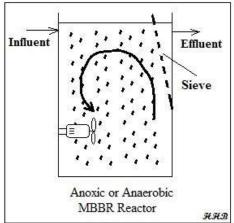
Initial Development of the MBBR Process: The MBBR process for wastewater treatment was invented and initially developed by Professor Hallvard Ødegaard in the late 1980s at the Norwegian University of Science and Technology. Use of this wastewater treatment process has spread rapidly. Per Ødegaard, 2014 (Reference #1 at the end of this course), there were already more than 800 MBBR wastewater treatment plants in more than 50 countries in 2014, with about half treating domestic wastewater and about half treating industrial wastewater. At least part of the reason for the interest in the MBBR process is its small footprint in comparison with other biological treatment processes. The tank volume needed for a MBBR process is typically significantly less than that needed for either an activated sludge process or a trickling filter designed to treat the same wastewater flow.

General Description of the MBBR Process: The MBBR process is an attached growth biological wastewater treatment process. That is, the microorganisms that carry out the treatment are attached to a solid medium, as in trickling filter or RBC systems. By contrast, in a suspended growth biological wastewater treatment process, like the activated sludge process, the microorganisms that carry out the treatment are kept suspended in the mixed liquor in the aeration tank.

In the conventional attached growth biological treatment processes, like trickling filter or RBC systems, the microorganisms are attached to a medium that is fixed in place and the wastewater being treated flows past the surfaces of the medium with their attached biological growth. In contrast, an MBBR process utilizes small plastic carrier media (described in more detail in the next section) upon which the microorganisms are attached. The MBBR treatment processes typically take place in a tank similar to an activated sludge aeration tank. The carrier media are kept suspended by a diffused air aeration system for an aerobic process or by a mechanical mixing system for an anoxic or anaerobic process, as illustrated in the figures below. A sieve is typically used at the MBBR tank exit to keep the carrier media in the tank.

Primary clarification is typically used ahead of the MBBR tank. Secondary clarification is also typically used, but there is no recycle activated sludge sent back into the process, because an adequate microorganism population is maintained attached to the media.





The MBBR Media Support Carrier System: MBBR processes use plastic media support carriers similar to those shown in the figure below. As shown in that figure, the carrier is typically designed to have a high surface area per unit volume, so that there is a lot of surface area on which the microorganisms attach and grow. Media support carriers like those shown in the figure are available from numerous vendors. Two properties of the carrier are needed for the process design calculations to be described and discussed in this course. Those properties are the specific surface area in m²/m³ and the void ratio. The specific surface area of MBBR carriers is typically in the range from 350 to 1200 m²/m³ and the void ratio typically ranges from 60% to 90%. Design values for these carrier properties should be obtained from the carrier manufacturer or vendor.

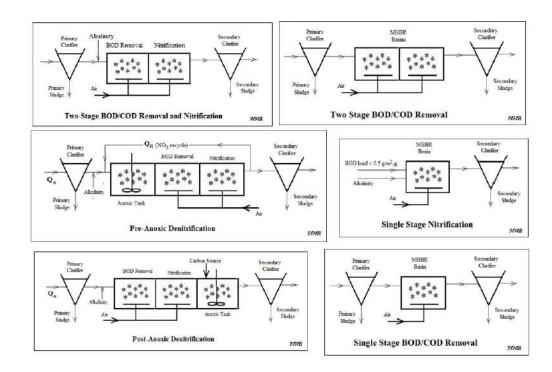


Typical MBBR Media Support Carriers

MBBR Wastewater Treatment Process Alternatives: The MBBR wastewater treatment process is quite flexible and can be used in several different ways. The figure below shows flow diagrams for the following six alternatives. Note that, as previously

mentioned, primary clarification and secondary clarification are shown for all of the process alternatives, but there is no sludge recycle as in a conventional activated sludge process. Also, note that a clarifier is not typically used between stages in a two or three-stage MBBR process.

- 1. Single stage BOD removal
- 2. Two stage BOD removal
- 3. Two stage BOD removal and Nitrification
- 4. Single stage tertiary Nitrification
- 5. Pre-Anoxic Denitrification
- Post-Anoxic Denitrification



Overview of MBBR Process Design Calculations: The key empirical design parameter used to determine the required MBBR tank size is the surface area loading rate (SALR) in g/m²/d. The g/d in the SALR units refers to the g/d of the parameter being removed and the m² in the SALR units refers to the surface area of the carrier. Thus, for BOD removal the SALR would be g BOD/day entering the MBBR tank per m² of carrier surface area. For a nitrification reactor, the SALR would be g NH₃-N/day entering the MBBR tank per m² of carrier surface area. Finally, for denitrification design, the SALR would be g NO₃-N/day per m² of carrier surface area.

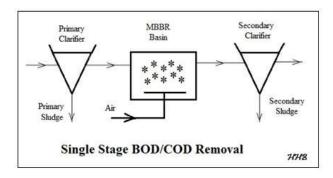
For any of these processes, a design value for SALR can be used together with design values of wastewater flow rate and BOD, ammonia or nitrate concentration, to calculate the required carrier surface area in the MBBR tank. The design carrier volume can then

be calculated using a known value for the carrier specific surface area (m²/m³). Finally, a design value for the carrier fill % can be used to calculate the required tank volume.

Process design calculations for each of the process alternatives shown in the figure above will be covered in the next several sections.

5. Single Stage BOD Removal MBBR Process Design Calculations

An MBBR single stage BOD removal process may be used as a free-standing secondary treatment process or as a roughing treatment prior to another secondary treatment process, in some cases to relieve overloading of an existing secondary treatment process. In either case the key design parameter for sizing the MBBR tank is the surface area loading rate (SALR), typically with units of g/m²/day, that is g/day of BOD coming into the MBBR tank per m² of carrier surface area. Using design values for wastewater flow rate and BOD concentration entering the MBBR tank, the loading rate in g BOD/day can be calculated. Then dividing BOD loading rate in g/day by the SALR in g/m²/day gives the required carrier surface area in m². The carrier fill %, carrier specific surface area, and carrier % void space can then be used to calculate the required carrier volume, tank volume and the volume of liquid in the reactor. A typical flow diagram for a single stage MBBR process for BOD removal is shown in the figure below.



The equations for making those calculations are as follows:

1. BOD loading rate = $Q*S_0*8.34*453.59$

where: Q is the wastewater flow rate into the MBBR reactor in MGD

So is the BOD concentration in that influent flow in mg/L
8.34 is the conversion factor from mg/L to lb/MG
453.59 is the conversion factor from lb to g
The calculated BOD loading rate will be in g/day.

2. required carrier surf. area = BOD Loading Rate/SALR

where: **BOD Loading Rate** is in g/day

SALR is the design surface area loading rate in g/m²/day The calculated **required carrier surface area** will be in m².

3. required carrier volume = required carrier surf. area/carrier specific surf. area

where: required carrier surface Area is in m²
carrier specific surface Area is in m²/m³
The calculated required carrier volume will be in m³.

4. required tank volume = required carrier volume/carrier fill %

where: required tank volume will be in the same units as required carrier volume.

5. liquid volume in tank

= required tank volume - [required carrier volume(1 - carrier % void space)]

where: all three volumes will be in the same units.

Note that volumes calculated in m³ can be converted to ft³ by multiplying by 3.2808³ ft³/m³.

Although hydraulic retention time (HRT) is not typically used as a primary design parameter for MBBR reactors, it can be calculated at the design wastewater flow rate, if the liquid volume in the tank is known. Also, if a design peak hour factor is specified, then the HRT at peak hourly flow can be calculated as well. The equations for calculating HRT are as follows:

1. Ave. $HRT_{des\ ave} = liquid\ vol.\ in\ tank*7.48)/[Q*10^6/(24*60)]$

where: liquid vol. in tank is in ft³

Q is in MGD

7.48 is the conversion factor for ft³ to gal
106 is the conversion factor for MG to gal
24*60 is the conversion factor for days to min

Ave. HRTdes ave will be in min

2. Ave. HRT_{peak hr} = Ave. HRT_{des ave}/Peak Hour Factor

where: Ave. HRTpeak hr will also be in min

Table 1 below shows typical SALR design values for BOD removal in MBBR reactors. Reference #2 at the end of this course (Ødegaard, 1999) is the source for the values in the table.

Table 1. Typical Design SALR Values for BOD Removal

Typical Design Values for MBBR reactors at 15°C							
Purpose	Treatment Target % Removal	Design SALR g/m²-d					
BOD Removal							
High Rate	75 - 80 (BOD ₇)	25 (BOD ₇)					
Normal Rate	85 - 90 (BOD ₇)	15 (BOD ₇)					
Low Rate	90 - 95 (BOD ₇)	7.5 (BOD ₇)					

Example #1: A design wastewater flow of 1.5 MGD containing 175 mg/L BOD (in the primary effluent) is to be treated in an MBBR reactor.

- a) What is the BOD loading rate to the reactor in g/day?
- b) What would be a suitable design SALR to use for a target of 90-95% removal?
- c) If the MBBR carrier has a specific surface area of 600 m²/m³ and design carrier fill % of 40%, what would be the required volume of carrier and required MBBR tank volume?
- d) If the design carrier % void space is 60%, what would be the volume of liquid in the MBBR reactor?
- e) If the design peak hour factor is 4, calculate the average hydraulic retention time at design average wastewater flow and at design peak hourly wastewater flow.

Solution:

- a) The BOD loading rate will be (1.5 MGD)(175 mg/L)(8.34 lb/MG/mg/L) = 2189 lb/day = (2189 lb/day)*(453.59 g/lb) =**993,000 g BOD/day**
- b) From **Table 1** above, a suitable design SALR value for BOD removal with a target BOD removal of 90–95% would be **7.5** g/m²/day
- c) Required carrier surface area = $(993,000 \text{ g/day})/(7.5 \text{ g/m}^2/\text{day}) = 132,403 \text{ m}^2$. Required carrier volume = $132,403 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = 220.7 m^3

For 40% carrier fill: Required tank volume = $220.7 \text{ m}^3/0.40 = 551.7 \text{ m}^3$.

- d) The volume of liquid in the reactor can be calculated as: tank volume [carrier volume(1 void %)], Thus the volume of liquid is: $551.7 [220.7(1 0.60)] = 463.4 \text{ m}^3 = 463.3*(3.2808^3) = 16,365 \text{ ft}^3$
- e) The HRT at design ave ww flow can be calculated as: $HRT_{des\ ave}$ = reactor liquid volume*7.48/[Q*10⁶/(24*60)] = 16,365*7.48/[1.5*10⁶/(24*60)] = 118 min

HRT_{peak hr} = HRT_{des ave}/peak hour factor = 118/4 = **29 min**

Use of an Excel Spreadsheet: The type of calculations just discussed can be done conveniently using an Excel spreadsheet. The screenshot on the next page shows an example Excel spreadsheet set up to make the calculations as just described for **Example #1**.

The worksheet shown in the screenshot is set up for user input values to be entered in the blue cells with the values in the yellow cells calculated by the spreadsheet using the equations presented and discussed above. The values calculated by the spreadsheet for the following parameters are the same as those shown in the solution to **Example #1** above: i) the BOD loading in g/day, ii) the required tank volume, iii) the liquid volume in the tank, iv) the hydraulic retention time at design average flow, and v) the hydraulic retention time at peak hourly flow.

You may have noticed that there are a few additional calculations in the worksheet screenshot. If the user selects "rectangular" for the tank shape, the worksheet calculates the tank length and width for the calculated required tank volume. If the user selects cylindrical as the tank shape, the worksheet will calculate the tank diameter. These calculations simply use the formulas for the volume of a rectangular tank (V = L^*W^*H) or for the volume of a cylindrical tank (V = $\pi D^2H/4$), with user entered values for L:W ratio being used for rectangular tank calculations and the user entered value for the liquid depth in the tank, H, being used by both. The worksheet also calculates an estimated effluent BOD concentration. The effluent BOD calculation is discussed in the next section.

Estimation of Effluent Concentration: Use of an estimated surface area removal rate (SARR) allows calculation of the estimated effluent concentration of the parameter being removed. That is, for BOD removal, the estimated effluent BOD concentration can be calculated. For nitrification, the estimated effluent ammonia nitrogen concentration can be calculated and for denitrification, the estimated effluent nitrate nitrogen concentration can be calculated.

Single-Stage Prod				74.			
Instructions: Enter valu	es in blue	boxes.	Sprea	dsheet calcul	ates values	in yellow	boxes
1. General Inputs							
		Ę.		Data points for S	SARR/SALR	s SALR	
Design ww Flow Rate, Q =	1.5	MGD		SALR (g	y/m²/d):	7.5	15.0
Prim. Effl. BOD, So =	175	mg/L		SARR/	SALR:	0.925	0.875
Peak Hour Factor =	Peak Hour Factor = 4				e are based removal vs S		
Design Value of BOD Surface			va		ARR/SALR vs	PER	-0.007
Area Loading Rate (SALR) =	7.5	g/m²/d			ARR/SALR VS		0.975
See information on typical of	was U.S.	g/m /u			of SARR/SAL		0.925
values for SALR below ri				(Surf. Area Ren (for SALR valu	noval Rate/Su	irf. Area Loa	2000000
2. Calculation of Carrier	Volume	and Re	quire	Tank Volun	ne & Dime	nsions	
Inputs				Liquid Depth in	Tank =	8	ft
Carrier Spec. Surf. Area =	600	m ² /m ³		Tank L:W	1.5		
(value from carrier mfr	(vendor)		(t	arget L:W - onl	y used if tan	k is rectang	jular)
Design Carrier Fill % = 40%				on green box a	and then on		
(Carrier fill % is typically between 30% and				w to Select Tar	nk Shape:	rectan	gular
70%. Lower values are more	e conserva	itive,		Carrier % Void	Space =	60%	
allowing future capacity exp	pansion or			(from carrier r	nfr/vendor -	only neede	d to
reduction of SALR by adding	g more car	rrier.)		calculate hy	draulic deter	ntion time)	
Calculations			-				
BOD Daily Loading =	2189	lb/day		Calculated Tan	k Volume =	551.7	m ³
	993,022	g/day			=	19,482	ft ³
					=	145,723	gal
Carrier Surf. Area needed =	132,403	m ²					
Calculated Carrier Volume =	220.7	m ³		Calculated Tar	nk Width =	40.3	ft
				Calculated Tar	nk Length =	60.4	ft
Tank Liquid Volume =	16,365	ft ³	Nor	minal Hydraulic F	Retention Time	e at	
				Design Aver	rage Flow =	118	min
Estimate of BOD Surface Are	а			Peak Ho	urly Flow =	29	min
Removal Rate, SARR =	6.94	g/m²/d					
Est. of BOD Removal Rate:	918,545	g/day		Calculated EffI I	BOD Conc.:	13	mg/L
	2025.1	lb/day		If the calcula	ted Effl. BOD	conc. is to	0
				high, the de	esign value o	of SALR	

Screenshot of MBBR Process Design Calculations – Single Stage BOD Removal

Based on graphs and tables provided in several of the references at the end of this document, the SARR/SALR ratio for all of the different types of MBBR treatment being covered in this course ranges from about 0.8 to nearly 1.0 over the range of SALR values typically used. The SARR/SALR ratio is nearly one at very low SALR values and decreases as the SALR value increases.

The upper right portion of the screenshot on the previous page illustrates an approach for estimating a value for the SARR/SALR ratio for a specified design value of SALR. In the four blue cells at the upper right, two sets of values for SARR/SALR and SALR are entered. In this case they are based on the typical values of % BOD removal vs SALR in Table 1 above. In the yellow cells below those entries, the slope and intercept of a SARR/SALR vs SALR straight line are calculated using the Excel SLOPE and INTERCEPT functions. Then the SARR/SALR ratio is calculated for the specified design value of SALR.

Note that the ratio SARR/SALR is equal to the % BOD removal expressed as a fraction. This can be shown as follows:

BOD removal rate in g/day = $(SARR in g/m^2/d)(Carrier Surf. Area in m^2)$

BOD rate into plant in g/day = (SALR in $g/m^2/d$)(Carrier Surf. Area in m^2)

% BOD removal = (BOD removal rate/BOD rate into plant)*100%

= (100%)(SARR* Carrier Surf Area)/(SALR*Carrier Surf Area)

= (SARR/SALR)100%

Thus, the value of **0.925** for the **SARR/SALR** ratio at **SALR = 7.5** g/m²/d was obtained from **Table 1** above as the midpoint of the 90-95% estimated % BOD removal for **SALR = 7.5** g/m²/d. Similarly, the value of **0.875** for the **SARR/SALR** ratio at **SALR = 15** g/m²/d was obtained from **Table 1** above as the midpoint of the 85-90% estimated % BOD removal for **SALR = 15** g/m²/d.

At the bottom of the screenshot worksheet, the estimated value of the surface area removal rate (SARR) is calculated. It is used to calculate an estimated BOD removal rate in g BOD/day and lb BOD/day. Then an estimate of the effluent BOD concentration is calculated. The equations for these calculations are as follows:

- 1. estimated SARR = (calculated SARR/SALR)(design value of SALR)
- 2. estimated BOD removal rate = (estimated SARR)(carrier surface area)
- 3. estimated effluent BOD conc.
 - = [(BOD loading rate estimated BOD removal rate)/Q₀]/8.34

Example #2: Calculate the estimated effluent BOD concentration for the wastewater flow described in **Example #1** being treated in the MBBR reactor sized in **Example #1**.

Solution: The solution is included in the spreadsheet screenshot that was used for the solution to **Example #1** above. The pair of points for **SARR/SALR vs SALR** that were discussed above and are shown on the screenshot lead to the following values for the slope and intercept for the **SARR/SALR vs SALR** line: Slope = -0.007, Intercept = 0.975.

Thus the estimated SARR/SALR ratio for the given SALR value of 7.5 g/m 2 /d would be calculated as: SARR/SALR = - (0.007)(7.5) + 0.975 = 0.925

The SARR value can be calculated as:

SARR =
$$(SARR/SALR)(SALR) = (0.925)(7.5) = 6.94 \text{ g/m}^2/\text{d}$$

Then, the estimated BOD removal rate can be calculated as:

est BOD removal rate = (est SARR)(carrier surface area)

$$= (6.94 \text{ g/m}^2/\text{d})(132,403 \text{ m}^2) = 918,545 \text{ g/d} = 918,545/453.59 \text{ lb/day}$$

est BOD removal rate = 2025.1 lb/day

The estimated effluent BOD concentration can then be calculated from the equation:

est effluent BOD conc. = [(BOD loading rate - est BOD removal rate)/Qo]/8.34

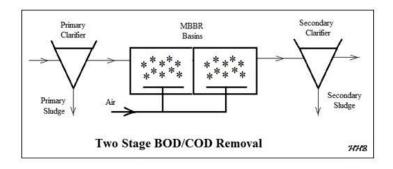
Substituting calculated and given values:

Est. effluent BOD conc. = [(2189 - 2026.1)/1.5]/8.34 = 13 mg/L

Note that this **13 mg/L** value for the **estimated effluent BOD** concentration is shown near the bottom of the spreadsheet screenshot.

6. Two-Stage BOD Removal MBBR Process Design Calculations

A two stage MBBR BOD removal process may be used instead of a single stage process. In this case, a high SALR "roughing" treatment will typically be used for the first stage and a lower SALR will typically be used for the second stage. This will result in less total tank volume needed for a two-stage process than for a single stage process. Also, a two-stage MBBR process can typically achieve a lower effluent BOD concentration than a single stage MBBR process. A typical flow diagram for a two-stage MBBR process for BOD removal is shown in the figure below.



Two Stage MBBR Process for BOD/COD Removal

The process design calculations for a two stage MBBR process are essentially the same for each of the stages as for the single stage process, as described in the previous section. These calculations are illustrated in **Example #3**.

Example #3: A design wastewater flow of 1.5 MGD containing 175 mg/L BOD (in the primary effluent) is to be treated for BOD removal in a two-stage MBBR reactor. The SALR for the first stage is to be 25 g/m²/d and the design SALR for the second stage is to be 7.5 g/m²/d.

- a) For the first stage calculate each of the following:
 - i) The BOD loading
 - ii) The required carrier volume for a carrier with specific surface area of 600 m²/m³
 - iii) The required MBBR tank volume for a design carrier fill % of 40%
 - iv) The volume of liquid in the MBBR reactor for design carrier % void space of 60%.
 - v) The average hydraulic retention time at design average wastewater flow and at design peak hourly flow if the design peak hour factor is 4.
 - vi) The estimated effluent BOD concentration from the first stage.
- b) Calculate the same parameters for the second stage.

Solution: The solution as calculated with an Excel spreadsheet is shown in the spreadsheet screenshot in the next two figures below. The first screenshot below which is the top part of the spreadsheet, shows primarily the user input values. It also includes the calculation of the slope and intercept for the SARR/SALR vs SALR equation and the calculation of the estimated SARR for each stage. These calculations and the resulting SARR/SALR values are the same as those discussed above for the single-stage BOD removal MBBR process. The resulting values for SARR/SALR are **0.775** for the first stage with SALR = **25**, and **0.925** for the second stage with SALR = **7.5**.

The second screenshot figure below is the bottom part of the spreadsheet and shows the calculated values as follows.

- a) For the first stage:
- i) The BOD loading rate will be (1.5 MGD)(175 mg/L)(8.34 lb/MG/mg/L) = 2189 lb/day = (2189 lb/day)*(453.59 g/lb) = 993,022 g BOD/day
- ii) Required carrier surface area = $(993,022 \text{ g/day})/(25 \text{ g/m}^2/\text{day}) = 39,721 \text{ m}^2$. Required carrier volume = $39,721 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = 66.20 m^3
- iii) For 40% carrier fill: Required tank volume = $66.2 \text{ m}^3/0.40 = \underline{165,5 \text{ m}^3}$.
- iv) The volume of liquid in the reactor can be calculated as: tank volume [carrier volume(1 void %)], Thus the volume of liquid is: $165.5 [66.20(1 0.60)] = 139.02 \, \text{m}^3$. = $139.02(3.2808^3) = 4910 \, \text{ft}^3$
- v) The HRT at design ave ww flow can be calculated as: HRT_{des ave} = reactor liquid volume*7.48/[Q*10⁶/(24*60)] = $4910*7.48/[1.5*10^6/(24*60)] = 35 \text{ min}$ HRT_{peak hr} = HRT_{des ave}/peak hour factor = 35/4 = 9 min
- vi) Calculation of the estimated effluent BOD concentration from the first stage as shown above for the single stage process gives a value of **39 mg/L**.
- b) For the second stage:
- i) The BOD loading rate will be (1.5 MGD)(39 mg/L)(8.34 lb/MG/mg/L) = 492.6 lb/day = (492.6 lb/day)*(453.59 g/lb) = 223,430 g BOD/day
- ii) Required carrier surface area = $(223,430 \text{ g/day})/(7.5 \text{ g/m}^2/\text{day}) = 29,791 \text{ m}^2$. Required carrier volume = $29,791 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = $\underline{49.65 \text{ m}^3}$
- iii) For 40% carrier fill: Required tank volume = $49.65 \text{ m}^3/0.40 = \underline{124.1 \text{ m}^3}$.
- iv) The volume of liquid in the reactor can be calculated as: tank volume [carrier volume(1 void %)], Thus the volume of liquid is: $124.1 [49.65(1 0.60)] = 104.3 \text{ m}^3 = 104.3(3.2808^3) = 3682 \text{ ft}^3$
- v) The HRT at design ave ww flow can be calculated as: HRT_{des ave} = reactor liquid volume*7.48/[Q*10⁶/(24*60)] = $3682*7.48/[1.5*10^6/(24*60)] = 26 \text{ min}$

 $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 26/4 = 7 min$

Calculation of the estimated effluent BOD concentration from the second stage using the calculation procedure shown above for the single stage process gives a value of **3.0 mg/L**.

MBBR Process Des Two-Stage Proces			Account of the control of the contro		
Instructions: Enter value	s in blue	boxes.	Spreadsheet calculates values	in yellow	boxes
I. Wastewater Parameter	Inputs				
1. Parameters for Both Fir	st and S	econd St	age		
Design ww Flow Rate, Q =	1.5	MGD	Peak Hour Factor =	4	
2. Parameters for First Sta	ge:		Data points for SARR/SALR v	s SALR	
			SALR (g/m²/d):	7.5	25.0
Prim. Effl. BOD, S _{o1} =	175	mg/L	SARR/SALR:	SARR/SALR: 0.925 0	
Design Value of BOD Surface			(default values above are based values of % BOD removal vs S		
Area Loading Rate (SALR) =	25	g/m ² /d	Slope, SARR/SALR vs	-0.009	
See information on typical de	esign	1,840	Intercept, SARR/SALR vs	SALR:	0.989
values for SALR below right.			Est. of SARR/SAL	R Rato =	0.775
			(Surf. Area Removal Rate/Su	rf. Area Loa	ading Rate)
3. Parameters for Second	Stage:				
Design Value of BOD Surface			Est. of SARR/SALR Rato =	0.925	
Area Loading Rate (SALR) =	7.5	g/m ² /d	(Surf. Area Removal Rate/Su	rf. Area Loa	ading Rate)
See information on typical de					-
values for SALR to the rig					
II. Carrier Parameter and	Tank S	hape In	outs for both First and Secon	nd Stage	<u>s</u>
Carrier Spec. Surf. Area =	600	m ² /m ³	Click on green box and then on		
(value from carrier mfr/v	/endor)		arrow to Select Tank Shape:		ngular
Liquid Depth in Tank =	8	ft	Carrier % Void Space =	60%	
Tank L:W ratio =	1.5		(from carrier mfr/vendor - o	only neede	ed to
(target L:W - only used if tan	k is recta	ngular)	calculate hydraulic deten	tion time)	

Screenshot of MBBR Process Design Calculations - Two Stage BOD Removal - Part 1

1. First Stage Calculation	s (BOD R	emoval)		(Carrier fill % is typically b	etween 30%	and 70%.	Lower
				values are more conservat	tive, allowi	ng future ca	apacity
Design Carrier Fill % =	40%	(for first stage) expansion or reduction of SALR by adding					
BOD Daily Loading =	2189	lb/day					
	993,022	g/day		Calculated Tank Volume =	165.5	m ³	
Carrier Surf. Area needed =	39,721	m ²		=	5844.7	ft ³	
Calculated Carrier Volume =	66.20	m ³		18	43718	gal	
Tank Liquid Volume =	4910	ft ³		Calculated Tank Width =	22.1	ft	
	139.0231	4909.374		Calculated Tank Length =	33.1	ft	
Estimate of BOD Surface Are	a		No	minal Hydraulic Retention Tin	ne at		
Removal Rate, SARR =	19.38	g/m²/d		Design Average Flow =	35	min	
Est. of BOD Removal Rate:	769,592	g/day		Peak Hourly Flow =	9	min	
	1696.7	lb/day		Calculated Effl BOD Conc.:	39	mg/L	
	"			(from First Stage)			
2. Second Stage Calculat Design Carrier Fill % =	40%	(for second	000	ge)			
BOD Daily Loading =	492.6	lb/day		Calculated Tank Volume =	124.1	m ³	
and the second property of the second propert	223,430	g/day		(E)	4383.5	ft ³	
Carrier Surf. Area needed =	29,791	m ²		y=	32789	gal	
Calculated Carrier Volume =	49.65	m ³		Calculated Tank Width =	19.1	ft	
Tank Liquid Volume =	3682	ft ³		Calculated Tank Length =	28.7	ft	
104.2673003	3682.031		No	minal Hydraulic Retention Tin	ne at		
Estimate of BOD Surface Are	a			Design Average Flow =	26	min	
Removal Rate, SARR =	6.94	g/m²/d		Peak Hourly Flow =	7	min	
Est. of BOD Removal Rate:	206,673	g/day		Calculated Effl BOD Conc.:	3.0	mg/L	
	455.6	lb/day		(from Second Stage)		-	
1st stage tank volume -			If th	e calculated Effl. BOD cond	. is too hig	h for either	S.
- 2nd stage tank volume =	41.4	0	stag	e, the design value of SALF	R should be	reduced	
					for that s	CONTRACTOR OF	

Screenshot of MBBR Process Design Calculations - Two Stage BOD Removal - Part 2

Example #4: Compare the MBBR tank volume, carrier surface area, and estimated effluent BOD concentration for the single stage BOD removal process in **Example #1** with the two-stage BOD removal process treating the same wastewater flow and influent BOD concentration as calculated in **Example #3**.

Solution: The results are summarized below:

Single Stage Process	Two-Stage Process
Cirigio Clago i 100000	1 WO Otago 1 100000

 MBBR Volume:
 19,482 ft³
 10,228 ft³

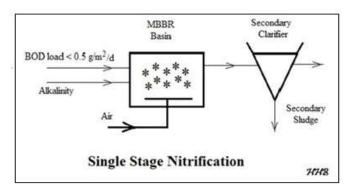
 Carrier Surf. Area:
 132,403 m²
 69,512 m²

 Est. Effl. BOD:
 13 mg/L
 3 mg/L

Note that the two-stage process requires only about half of the tank volume and half of the carrier quantity in comparison with the single stage process, while achieving a significantly lower estimated effluent BOD.

7. Single Stage Nitrification MBBR Process Design Calculations

An MBBR single stage nitrification process would typically be used as a tertiary treatment process following some type of secondary treatment that reduced the BOD to a suitable level. A typical flow diagram for a single stage MBBR process for nitrification is shown in the figure below. As shown on the diagram, the BOD level should be low enough so that the BOD load to the nitrification process is less than 0.5 g/m²/day. Note that alkalinity is used in the nitrification process and thus alkalinity addition is typically required.



Single Stage MBBR Process for Nitrification

The process design calculations for this single stage MBBR process are similar to those used for the BOD removal processes, but the design SALR value for nitrification can be calculated rather than being selected from a table of typical values, as was done for BOD removal. The design SALR can be calculated using a kinetic model for the surface area removal rate (SARR) as a function of the dissolved oxygen concentration in the MBBR reactor and the bulk liquid ammonia nitrogen concentration, which is equal

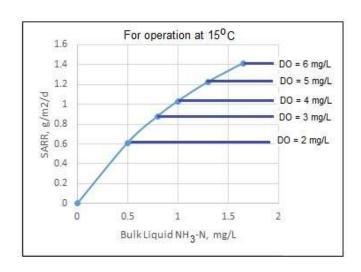
to the effluent ammonia nitrogen concentration assuming completely mixed conditions in the MBBR tank.

The kinetic model to be discussed here is from Metcalf and Eddy (2014), Figure 9-25 [attributed to Odegaard (2006)] and Equation 9-48. This figure and equation will now be shown and discussed briefly. The figure below was prepared based on Metcalf and Eddy's Figure 9-25 and their Equation 9-48 (shown below). Note that the figure and equation are for operation at 15°C. Correction of the SARR and SALR for some other operational temperature can be done with the equation: SARR_T = SARR₁₅ $\theta^{(T-15)}$ where T is the design operational temperature in °C. From Salvetti, et.al (2006): θ = **1.058** for D.O. limited conditions and θ = **1.098** if ammonia nitrogen concentration is the limiting factor.

Equation 9-48 from Metcalf and Eddy (2014) is:

SARR =
$$[N_e/(2.2 + N_e)]*3.3$$
 , N_e = effluent ammonia N conc.

This is the equation for the curved portion at the left in the graph below, which is for ammonia N concentration limiting conditions.



Adapted from Metcalf & Eddy (2014), Fig 9-25

The horizontal lines in the figure above show the nitrification **SARR** under D.O. limiting conditions for each of the D.O. levels shown. The **SARR** will be D.O. limited when the **NH₃-N** concentration is above the value at the left end of the horizontal line for each D.O. level. When the **NH₃-N** concentration is below that value, then the **SARR** is ammonia concentration limited and the **SARR** is a function of the effluent **NH₃-N** concentration (N_e) per the equation: **SARR = [** $N_e/(2.2 + N_e)]^*3.3$.

As shown in **Section 5** for BOD removal, **SALR/SARR = % BOD removal**. Similarly, for nitrification, **SALR/SARR = % NH₃-N removal**. After the **SARR** has been determined, the **SALR** can be calculated as: **SALR = SARR/% removal**.

The maximum **SARR** for each of the D.O. levels shown in the figure above are shown in **Table 2** below, along with the ammonia nitrogen concentration above which the **SARR** will be at that maximum value.

Table 2. Values of SARR_{max} and NH₃-N_e @ SARR_{max}

D.O.	SARR _{max}	min NH ₃ -N _e @ SARR _{max}
mg/L	g/m²/d	mg/L
2	0.61	0.5
3	0.88	0.8
4	1.03	1
5	1.23	1.3
6	1.41	1.65

Example #5: A design flow of 0.2 MGD has the following characteristics: 15 mg/L BOD, 25 mg/L NH₃-N, and alkalinity of 140 mg/L as CaCO₃. This flow is to be treated in a single stage nitrification MBBR reactor. The target effluent NH₃-N is 3.3 mg/L. The dissolved oxygen is to be maintained at 3.0 mg/L in the MBBR reactor. The design minimum wastewater temperature is to be 45°F.

- a) Determine an appropriate NH₃-N surface area loading rate (SALR) for this nitrification process, in g NH₃-N/m²/d.
- b) What is the NH₃-N loading rate to the reactor in g/day?
- c) If the MBBR carrier has a specific surface area of 600 m²/m³ and design carrier fill % of 40%, what would be the required volume of carrier and required MBBR tank volume?
- d) Calculate the BOD surface area loading rate (SALR) to ensure that it is less than 0.5 g BOD/m²/day.
- e) If the design carrier % void space is 60%, what would be the volume of liquid in the MBBR reactor.
- f) If the design peak hour factor is 4, calculate the average hydraulic retention time at design average wastewater flow and at design peak hourly wastewater flow.
- g) Calculate the alkalinity requirement in lb/day as CaCO₃ and in lb/day NaHCO₃ for a target effluent alkalinity of 80 mg/L.

Solution:

The solution is shown in the spreadsheet screenshots below. A summary of the calculations is as follows:

a) The D.O. limited **SARR** can be obtained as **SARR**_{max} from **Table 2** for the specified D.O level, and the minimum ammonia nitrogen concentration for that **SARR** value can be obtained from the same table. The values from **Table 2**, for a D.O. of 3.0 are: **SARR**_{max} = **0.88** g/m²/d and minimum **NH**₃-**N**_e for that value of **SARR**_{max} = **0.80** mg/L. (in the worksheet shown in the first screenshot below, these two values are obtained using Excel's VLOOKUP function from a table like **Table 2**, above that is on the worksheet.

The **SARR** for the design D.O. and ammonia nitrogen removal at 15°C will then be equal to **SARR**_{max} if the target effluent ammonia nitrogen concentration is greater than the 0.80 mg/L value determined above. If the target effluent ammonia nitrogen concentration is less than 0.80 mg/L, then the SARR needs to be calculated using Metcalf & Eddy's equation 9-48. In this case, the target effluent NH₃-N of 3.3 mg/L is greater than 0.8 mg/L, so the SARR at 15°C is 0.88 g/m²/d.

The design value for the SARR at the design minimum wastewater temperature can then be calculated as: SARR_T = SARR₁₅ $\theta^{(T-15)}$, where the WW temperature must be in °C. Since this case has D.O. limited conditions, θ = 1.058. Carrying out this calculation gives: **design value of SALR** = 0.65 g/m²/d.

- b) The ammonia nitrogen loading rate will be (0.2 MGD)(25 mg/L)(8.34 lb/MG/mg/L) = 41.7 lb/day = (41.7 lb/day)*(453.59 g/lb) = 18,915 g NH₃-N/day
- c) Required carrier surface area = $(18,915 \text{ g/day})/(0.65 \text{ g/m}^2/\text{day}) = 28,925 \text{ m}^2$.

Required carrier volume = $28,925 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = 48.209 m^3 = $(48.209 \text{ m}^3)(3.2808^3 \text{ ft}^3/\text{m}^3)$ = 1702 ft^3 .

For 40% carrier fill: Required tank volume = $1702 \text{ ft}^3/0.40 = \underline{4256 \text{ ft}^3}$.

- d) The BOD SALR will be $(0.2 \text{ MGD})(15 \text{ mg/L})(8.34 \text{ lb/MG/mg/L})(453.59)/(28925 \text{ m}^2) = 0.39 \text{ g/m}^2/\text{day}$ (Note that this is less than 0.5 g/m²/day as required.)
- e) The volume of liquid in the reactor can be calculated as: tank volume [carrier volume(1 void %)], Thus the volume of liquid is: $4256 [1702(1 0.60)] = 3575 \text{ ft}^3$.

f) The HRT at design ave ww flow can be calculated as: $HRT_{des\ ave} = reactor\ liquid\ volume^7.48/[Q^10^6/(24^60)] = 3575^7.48/[0.2^10^6/(24^60)] = 193\ min$

 $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 193/4 = 48 min$

g) Calculation of the alkalinity requirement is shown in the second screenshot below. The alkalinity needed for nitrification is 7.14 g CaCO₃/g NH₃-N removed. Thus, the alkalinity to be added in mg/L can be calculated as:

7.14(mg/L NH₃-N removed) + target effl. Alkalinity – influent alkalinity

For the given input values, this becomes:

$$7.14(25-3.3) + 80 - 140 = 94.9 \text{ mg/L as CaCO}_3$$

The rate of alkalinity addition needed can then be calculated as:

$$(0.2 \text{ MGD})(94.9 \text{ mg/L})*8.34 = 158.4 \text{ lb/day as CaCO}_3.$$

Multiplying this by the ratio of the equivalent weight of NaHCO₃ (84) to the equivalent weight of CaCO₃ (50) gives the daily NaHCO₃ requirement as

266.0 lb/day NaHCO₃.

	ary relui	ication	FIOCE	:55			
Instructions: Enter valu	es in blu	e boxes.	Sprea	dsheet calculates values	in yellow bo	xes	
1. General Inputs				Peak Hour Factor =	4		
			Target	Effl NH ₄ -N Conc, NH ₄ -N _e =	3.3	mg/L	
Design ww Flow Rate, Q =	0.2	MGD	12000	Min Design Temp., T =	45	°F	
Influent NH ₄ -N Conc. =	25	mg/L				10	
Influent BOD, So =	15	mg/L		Click on cell H10 and on	arrow to sele	ect D.O C	
Prim. Effl. Alkalinity =	140	mg/L as	CaCO ₃	D.O Conc. in Reactor =	3.0	mg/L	
2. Preliminary Calculati	ons - De	sign SA	LR val	u <u>e</u>			
% NH ₄ -N removal =	87%			NH ₄ -N _e @ SARR _{max} =	0.80	mg/L	
Maximum SARR =	0.88	g/m ² /d	SARR	@ NH ₄ -Ne, 15°C, SARR ₁₅ =	0.88	g/m²/d	
		100	E-Rossieson	@ NH ₄ -Ne, T °C, SARR _T =	0.57	g/m²/d	
SARR Temp. Coeff, 6 =	1.058				* * **********************************		
	1135550			Design Value for SALR =	0.65	g/m²/c	
Carrier Spec. Surf. Area = (value from carrier mfr	600 /vendor)	m ² /m ³		Liquid Depth in Tank = Tank L:W ratio =	8 1.5	ft	
			(t	arget L:W - only used if tank		ar)	
Design Carrier Fill % =	40%		Click	on green box and then on	7		
(Carrier fill % is typically be	tween 30%	and	especial	w to Select Tank Shape:	rectan	gular	
70%. Lower values are mor			Carrier % Void Space = 60%				
allowing future capacity ex		-	(from carrier mfr/vendor - only needed to				
reduction of SALR by addin				calculate hydraulic deter	ition time)		
		•					
Calculations						201	
				Calculated Tank Volume =	120.5	m ³	
NH ₃ -N Daily Loading =	41.7	lb/day		=	4256.2	ft ³	
	18915	g/day		=	31837	gal	
	28925	m ²		Calculated Tank Width =	18.8	ft	
Carrier Surf. Area needed =	48.209	m ³		Calculated Tank Length =	28.2	ft	
	40.203						
	3575.2	ft ³	Nor	ninal Hydraulic Retention Time	at		
Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume =	22442	ft ³	Nor	Design Average Flow =	193	min	

Screenshot of MBBR Process Design Calculations – Single Stage Nitrification – Part 1

5. Alkalinity Requ	irement							
Input:	T	arget Efflue	ent Alkalinity	=	80	mg/L		
Constants needs	d for Ca	lculation	is:					
Equiv Wt. of CaC	:O ₃ =	50	g/equiv.		Equiv Wt.	of NaHCO ₃ =	84	g/equiv.
Alkalinity used	for Nitrifica	ation =	7.14	g Cat	CO ₃ /g NH ₃ -N			
Calculations								
Alkalinit	y to be ad	lded =	94.9	mg/L	as CaCO ₃			
Daily Alkalin	ity Require	ement =	158.4	lb/da	y as CaCO ₃			
For sodium bica	arbonate	use to ad	d alkalinity					
Daily NaHCO	3 Require	ment =	266.0	lb/da	y NaHCO ₃			

Screenshot of MBBR Process Design Calculations – Single Stage Nitrification – Part 2

8. Two-Stage BOD Removal and Nitrification MBBR Process Design Calculations

A two stage MBBR process may also be used to achieve both BOD removal and nitrification. Nitrification with an MBBR process requires a rather low BOD concentration in order to favor the nitrifying bacteria in the biomass attached to the carrier. Thus, the first stage for this process is used for BOD removal and the second stage is used for nitrification. A typical flow diagram for a two stage MBBR process for BOD removal and nitrification is shown in the figure below. As in the single stage nitrification process alkalinity is used for nitrification, so alkalinity addition is typically required.

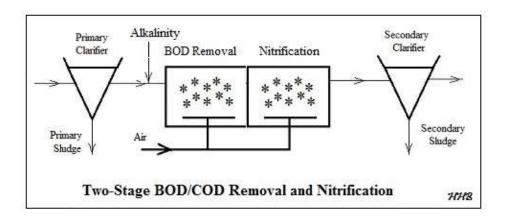


Table 3 shows typical design values for the SALR (surface area loading rate) for the BOD removal stage and for the nitrification stage. The source for the values in this table is the Odegaard reference #2 below. The design SALR value for nitrification, however, will be calculated based on the design D.O concentration and the target effluent NH₃-N concentration, as it was for the single-stage nitrification process.

Table 3. Typical Design SALR Values for Nitrification

Purpose	Treatment Target % Removal	Design SALR g/m ² -d
Nitrification	00 05 (BOD.)	6 A /BOD \
BOD removal stage Effl. NH ₃ -N > 3 mg/L	90 - 95 (BOD ₇) 90 (NH ₃ -N)	6.0 (BOD ₇) 1.00 (NH ₃ -N)
Effl. NH ₃ -N < 3 mg/L	90 (NH ₃ -N)	0.45 (NH ₃ -N)

The process design calculations for this two stage MBBR process are essentially the same as those described above for the previous examples. **Table 3** will be used to obtain values for SARR/SALR vs SALR for the BOD removal stage as in the previous examples. The design SALR for the nitrification stage will be calculated as it was for the single stage nitrification process.

The process design calculations for this two-stage process are illustrated in **Example** #6

Example #6: A design wastewater flow of 1.5 MGD has the following primary effluent characteristics: 175 mg/L BOD, 35 mg/L TKN, and alkalinity of 140 mg/L as CaCO₃. The design minimum wastewater temperature is 45°F. This flow is to be treated for BOD removal and nitrification in a two stage MBBR process. The design SALR for the first stage is to be 6 g BOD/m²/d, the target effluent NH3-N from the second stage is to be 3.3 mg/L and the design DO level in the nitrification stage is to be 3.0 mg/L.

- a) For the first (BOD removal) stage calculate each of the following:
 - i) The BOD loading
 - ii) The required carrier volume for a carrier with specific surface area of 600 m²/m³
 - iii) The required MBBR tank volume for a design carrier fill % of 40%

- iv) The volume of liquid in the MBBR reactor for design carrier % void space of 60%.
- v) The average hydraulic retention time at design average wastewater flow and at design peak hourly flow if the design peak hour factor is 4.
- vi) The estimated effluent BOD concentration from the first stage.
- b) For the second (Nitrification) stage calculate each of the following:
 - i) An appropriate NH₃-N surface area loading rate (SALR) to be used for this nitrification process, in g NH₃-N/m²/d.
 - ii) The nitrate loading
 - iii) The required carrier volume for a carrier with specific surface area of 600 m²/m³
 - iv) The required MBBR tank volume for a design carrier fill % of 40%
 - v) The volume of liquid in the MBBR reactor for design carrier void space of 60%.
 - vi) The average hydraulic retention time at design average wastewater flow and at design peak hourly flow if the design peak hour factor is 4.
 - vii) The alkalinity requirement in lb/day as CaCO₃ and in lb/day NaHCO₃.

Solution: The solution, as calculated with an Excel spreadsheet, is shown in the three spreadsheet screenshots on the next several pages. The first screenshot, which is the top part of the spreadsheet, shows primarily the user input values. It includes the calculation of the slope and intercept for the SARR/SALR vs SALR equation and the calculation of the estimated SARR for the BOD removal stage. It also includes calculation of the design SALR value for the nitrification stage. These calculations are carried out as discussed above for the single-stage BOD removal and nitrification MBBR processes. The resulting values for SARR/SALR are **0.935** for the first stage with SALR = **6** g BOD/m²/d, and **0.63** g/m2/d for the second stage (nitrification) SALR.

The second screenshot is the bottom part of the spreadsheet and shows the calculated values as follows.

- a) For the first stage:
- i) The BOD loading rate will be (1.5 MGD)(175 mg/L)(8.34 lb/MG/mg/L) = 2,189 lb/day = (2189 lb/day)*(453.59 g/lb) =**993,022 g BOD/day**
- ii) Required carrier surface area = $(993022 \text{ g/day})/(6 \text{ g/m}^2/\text{day}) = 165,504 \text{ m}^2$. Required carrier volume = $165,504 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = 275.8 m^3
- iii) For 40% carrier fill: Required tank volume = $275.8 \text{ m}^3/0.40 = \underline{689.6 \text{ m}^3}$.
- iv) The volume of liquid in the reactor can be calculated as: tank volume [carrier volume(1 void %)], Thus the volume of liquid is: $689.6 [275.8(1 0.60)] = 579.3 \,\text{m}^3 = 579.3(3.2808^3) = 20,456 \,\text{ft}^3$.

v) The HRT at design ave ww flow can be calculated as: HRT_{des ave} = reactor liquid volume*7.48/[Q*10⁶/(24*60)] = $20,456*7.48/[1.5*10^6/(24*60)] = 147 \text{ min}$

 $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 147/4 = 37 min$

Instructions: Enter valu	es in blue	boxes.	Spread	sheet calcula	tes values ir	yellow b	oxes
I. Wastewater Paramete	r Inputs						
1. Parameters for Both Fi	rst and Se	cond St	age				
Design ww Flow Rate, Q =	1.5	MGD		Peak Hour	Factor =	4	
		Pr	rim. Effl. /	Alkalinity =	140	mg/L as (CaCO ₃
2. Parameters for First St	age:			Data points for	SARR/SALR vs	SALR	
**				SALR (g/m²/d):	7.5	15.0
Prim. Effl. BOD, So1 =	175	mg/L		SARR/		0.925	0.875
Design Value of BOD Surface				t values above ies of % BOD r			
Area Loading Rate (SALR) =	6	g/m ² /d		Slope, SARR/SALR vs SALR:			
See information on typical d	esian			A	ARR/SALR vs \$	00000000000	0.975
values for SALR at the rig					SARR/SALR R	MODEL CONTRACTOR OF THE PARTY O	0.935
				(Surf. Area Rer	noval Rate/Sur	f. Area Load	ding Rate)
3. Parameters for Second	Stage:						
Influent NH ₄ -N Conc. =	35	mg/L	Target B	Effl NH ₄ -N Conc	, NH ₄ -N _e =	3.3	mg/L
Min Design Temp., T =	45	°F	Click	on cell H26 ar	nd on arrow to	select D.	O Conc.
				D.O Conc	in Reactor =	3.0	mg/L
4. Preliminary Calculation	ns - Des	ign Nitr	ificatio	n SALR valu	<u>ie</u>		
% NH ₄ -N removal =	91%			NH ₄ -N _e @ SA	RR _{max} =	0.80	mg/L
Maximum SARR =	0.88	g/m ² /d	SARR (@ NH ₄ -Ne, 15°0	C, SARR ₁₅ =	0.88	g/m ² /d
			SARR (@ NH ₄ -Ne, T °C	, SARR _T =	0.57	g/m²/d
SARR Temp. Coeff, 0 =	1.058	D	esign Va	lue for nitrificati	on SALR =	0.63	g/m²/d
II. Carrier Parameter and	d Tank Si	mape Ing				Stages	
Carrier Spec. Surf. Area = (value from carrier mfr/	100000	301.700		n green box a to Select Tan		recta	ngular
Liquid Depth in Tank =	8	ft	anow	Carrier % Voic	1500 CO 100 CO 1	60%	guiui
Tank L:W ratio =	1.5	- 55		(from carrier		7.7.7.2.2	d to
(target L:W - only used if ta	Access to the second	hallan		calculate hy			

Screenshot of MBBR Process Design Calculations for Two Stage BOD Removal and Nitrification – Part 1

- vi) Calculation of the estimated effluent BOD concentration from the first stage as shown above for the single stage BOD removal process gives a value of **11 mg/L**.
- b) For the second stage:
- i) The design value for the nitrification **SALR** is **0.63 g/m²/d**, calculated in the same way as it was for the single stage nitrification process.

1. First Stage Calculation	s - BOD R	Removal	(Carrier fill % is typically be	tween 30%	and 70%. Lower
			values are more conservati	ve, allowin	g future capacity
Design Carrier Fill % =	40%	(for first stage)	expansion or reduction of 5	SALR by add	ding more carrie
BOD Daily Loading =	2189.3	lb/day			
	993022	g/day	Calculated Tank Volume =	689.6	m ³
Carrier Surf. Area needed =	165503.7	m ²	1 = 1	24352.9	ft ³
Calculated Carrier Volume =	275.839	m ³	(=	182160	gal
Tank Liquid Volume =	20456.5	ft ³	Calculated Tank Width =	45.0	ft
			Calculated Tank Length =	67.6	ft
Estimate of BOD Surface Are	ea	Nor	minal Hydraulic Retention Time	at	
Removal Rate, SARR =	5.61	g/m²/d	Design Average Flow =	147	min
Est. of BOD Removal Rate:	928475	g/day	Peak Hourly Flow =	37	min
	2046.9	lb/day	Calculated Effl BOD Conc.:	11	mg/L
			(from First Stage)		
If the calculated Effl. BOD of	conc. is too	high, the design		e reduced i	n cell C18.
If the calculated Effl. BOD of	conc. is too	high, the design	value of the SALR should b	e reduced i	n cell C18.
				e reduced i	n cell C18.
				e reduced i	n cell C18.
2. Second Stage Calculat			value of the SALR should b		
2. Second Stage Calculat	tions - Nitr	ification	value of the SALR should b	1320.5	m ³
2. Second Stage Calculat	tions - Nitr	ification (for second stage	Calculated Tank Volume =	1320.5 46632.1	m³ ft³
2. Second Stage Calculat Design Carrier Fill % = NH ₃ -N Daily Loading =	40% 437.9	(for second stage	Calculated Tank Volume = e) = Calculated Tank Width =	1320.5 46632.1 348808	m³ ft³ gal
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2. Second Stage Calculate Design Carrier Fill % = NH ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = 1st stage tank volume -	40% 437.9 198604 316914.0 528.190 42736	(for second stage lb/day g/day m² Nor ft³	Calculated Tank Volume = e) = Calculated Tank Width = Calculated Tank Width = Calculated Tank Length = minal Hydraulic Retention Time Design Average Flow = Peak Hourly Flow =	1320.5 46632.1 348808 62.3 93.5 at 307 77	m ³ ft ³ gal ft ft min min

Screenshot of MBBR Process Design Calculations for Two Stage BOD Removal and Nitrification – Part 2

- ii) The NH₃-N loading rate will be (1.5 MGD)(35 mg/L)(8.34 lb/MG/mg/L) = 437.9 lb/day = (437.9 lb/day)*(453.59 g/lb) = 198,604 g NH₃-N/day
- iii) Required carrier surface area = $(198,604 \text{ g/day})/(0.63 \text{ g/m}^2/\text{day})$ = 316,914 m².

Required carrier volume = $316914 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = 528.19 m^3 = $(528.19 \text{ m}^3)(3.2808^3 \text{ ft}^3/\text{m}^3)$ = $18,652 \text{ ft}^3$.

- iv) For 40% carrier fill: Required tank volume = $18652 \text{ ft}^3/0.40$ = $46,632 \text{ ft}^3$.
- v) The volume of liquid in the reactor can be calculated as: tank volume [carrier volume(1 void %)], Thus the volume of liquid is: 46632 [18652(1 0.60)] = **42,736** ft³.
- vi) The HRT at design ave ww flow can be calculated as: $HRT_{des\ ave} = reactor\ liquid\ volume*7.48/[Q*10^6/(24*60)] = 42736*7.48/[1.5*10^6/(24*60)] = 307\ min$

 $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 307/4 = 77 min$

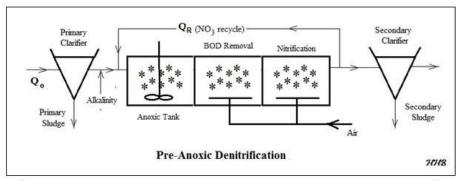
Input:	Tar	get Effluen	t Alkalinity =		80	mg/L		
Constants neede	d for Cal	culation	<u>s:</u>					
Equiv Wt. of CaC	O ₃ =	50	g/equiv.		Equiv Wt.	of NaHCO ₃ =	84	g/equiv.
Alkalinity used	for Nitrificat	tion =	7.14	g CaC	O ₃ /g NH ₃ -N			
Calculations								
Alkalinit	y to be add	ded =	166.3	mg/L	as CaCO ₃			
Daily Alkalini	ty Require	ment =	2080.9	lb/day	as CaCO ₃			
For sodium bica	ırbonate ι	ise to add	l alkalinity:					
Daily NaHCO	3 Requiren	nent =	3495.9	lb/dav	NaHCO ₃			

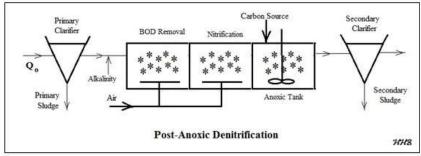
Screenshot of MBBR Process Design Calculations for Two Stage BOD Removal and Nitrification – Part 3

vii) Calculation of the alkalinity requirement is shown in part 3 of spreadsheet screenshot, shown above. Using the equivalent weight of CaCO₃ as 50, the equivalent weight of NaHCO₃ as 84, the alkalinity use for nitrification as 7.14 g CaCO₃/g NH₃-N and the target effluent alkalinity as 80 mg/L as CaCO₃, give the calculated alkalinity requirement as 166.3 mg/L as CaCO₃. The rate of alkalinity addition needed can then be calculated as: (1.5 MGD)(166.3 mg/L)*8.34 = 2080.9 lb/day as CaCO₃. Multiplying this by the ratio of the equivalent weight of NaHCO₃ to the equivalent weight of CaCO₃ gives the daily NaHCO₃ requirement as 3495.9 lb/day NaHCO₃.

9. Denitrification Background Information

In order to carry out denitrification of a wastewater flow (removal of the nitrogen from the wastewater), it is necessary to first nitrify the wastewater, that is, convert the ammonia nitrogen typically present in the influent wastewater to nitrate. Nitrification will only take place at a reasonable rate in an MBBR reactor if the BOD level is quite low. Thus, an MBBR denitrification process will need a reactor for BOD removal, one for nitrification, and one for denitrification. The nitrification reactor will always follow the BOD removal reactor, because of the need for a low BOD level in the nitrification reactor. The denitrification reactor may be either before the BOD removal reactor (called pre-anoxic denitrification) or after the nitrification reactor (called post-anoxic denitrification). Flow diagrams for these two denitrification options are shown in the figure below.





MBBR Denitrification Process Alternatives

A bit more information about the denitrification reactions will be useful for further discussion of these two denitrification options. The denitrification reactions, which convert nitrate ion to nitrogen gas, and hence remove it from the wastewater flow, will take place only in the absence of oxygen, that is, in an anoxic reactor. Also, the denitrification reactions require a carbon source. With these factors in mind the functioning of the pre-anoxic denitrification process and of the post-anoxic denitrification process are described in the following two paragraphs.

In a **pre-anoxic denitrification** process, the BOD in the primary effluent wastewater is used as the carbon source for denitrification. In this process, however, the primary effluent entering the pre-anoxic reactor still has ammonia nitrogen present, rather than the nitrate nitrogen needed for denitrification. A recycle flow of effluent from the nitrification reactor is used to send nitrate nitrogen to the anoxic denitrification reactor as shown in the flow diagram above.

In a **post-anoxic denitrification** process, the influent to the denitrification reactor comes from the nitrification reactor, so the wastewater influent ammonia nitrogen has been converted to nitrate as required for denitrification. The BOD has also been removed prior to the post anoxic denitrification reactor, however, so an external carbon source is required for the denitrification reactions. Methanol is a commonly used carbon source for post-anoxic denitrification.

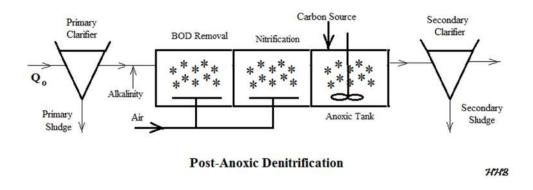
The pre-anoxic denitrification process has the advantage of not requiring an external carbon source and it reduces the BOD load to the BOD removal part of the process, because BOD is used in the denitrification reactions. However, the pre-anoxic process requires an influent C/N ratio greater than 4, where C/N is taken to be BOD/TKN, and the post-anoxic process can achieve a more complete nitrogen removal.

From the Odegaard references (#5 and #6 at the end of the course) suitable criteria for determining whether to use pre- or post-anoxic denitrification are as follows:

- Pre-anoxic denitrification is suitable if C/N ≥ 4 and target % Removal of N < 75%
- Post-anoxic denitrification should be used if C/N < 4 or target % Removal of N > 75%

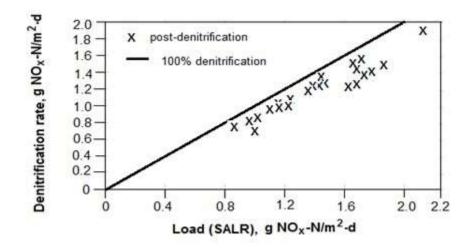
10. Post-Anoxic Denitrification Process Design Calculations

Process design of a post-anoxic denitrification MBBR system, requires sizing an MBBR tank for BOD removal, one for nitrification and one for denitrification. For all three of these reactors the key design parameter for sizing the MBBR tank is the surface area loading rate (SALR), typically with units of g/m²/day, that is g/day (of the parameter being removed in that reactor) coming into the MBBR tank per m² of carrier surface area in the reactor. Using design values for wastewater flow rate and concentration of the removed parameter entering the MBBR tank, the loading rate in g/day can be calculated. Then dividing the loading rate in g/day by the SALR in g/m²/day gives the required carrier surface area in m². The carrier fill %, carrier specific surface area, and carrier % void space can then be used to calculate the required carrier volume, tank volume and the volume of liquid in the reactor. A typical flow diagram for a post-anoxic denitrification MBBR process is shown in the figure on the next page.



MBBR Flow Diagram for Post-Anoxic Denitrification

Process design calculations for the BOD removal stage and the nitrification stage will be done just as described in **Section 8**, **Two-Stage BOD Removal and Nitrification Process Design Calculations**, and illustrated with the examples in that section. The process design calculations for denitrification, which is the third stage in a post-anoxic denitrification process, are similar to those previously discussed and illustrated for the BOD removal stage and the nitrification stage. The graph shown in the figure below (prepared using values from a similar graph in Rusten and Paulsrud's presentation in Ref #4 at the end of this course) will be used to obtain values for SARR/SALR vs SALR for the denitrification stage.



SARR vs SALR for Post-Anoxic Denitrification

Table 4 below shows typical SALR design values for pre-anoxic denitrification and post-anoxic denitrification in MBBR reactors. Reference #2 at the end of this book is the source for the values in **Table 4**.

Table 4. Typical Design SALR Values for Denitrification

Typical Design Values for MBBR reactors at 15°C					
Purpose	Treatment Target % Removal	Design SALR g/m²-d			
Denitrification					
Pre-DN (C/N > 4)	70 (NO ₃ -N)	0.90 (NO ₃ -N)			
Post-DN (C/N > 3)	90 (NO ₃ -N)	2.00 (NO ₃ -N)			

Example #7: A design wastewater flow of 1.5 MGD containing 175 mg/L BOD, 35 mg/L TKN, and alkalinity of 140 mg/L as CaCO₃ (in the primary effluent) is to be treated in a post-anoxic denitrification MBBR process. The design SALR for the first stage is to be 6 g BOD/m²/d. For the second stage, the SALR should be calculated for a target effluent NH₄-N conc. of 3.3 mg/L, min. design temperature of 45°F, and D.O. in reactor of 3.0 mg/L. For the post-anoxic stage, the SALR is to be 2 g NO₃-N/m²/d and the target effluent NO₃-N conc. is to be 5.0 mg/L.

For the third (Denitrification) stage calculate each of the following:

- i) The nitrate loading
- ii) The required carrier volume for a carrier with specific surface area of 600 m²/m³
- iii) The required MBBR tank volume for a design carrier fill % of 40%
- iv) The volume of liquid in the MBBR reactor for design carrier % void space of 60%.
- v) The average hydraulic retention time at design average wastewater flow and at design peak hourly flow if the design peak hour factor is 4.
- vi) The estimated effluent NO₃-N concentration from the denitrification stage.
- vii) The alkalinity requirement in lb/day as CaCO₃ and in lb/day NaHCO₃, for target effluent alkalinity of 80 mg/L as CaCO₃.
- viii) The methanol requirement in lb/day for methanol use as the carbon source.

Note that the process design calculations for the BOD removal stage and the nitrification stage of this process will be the same as those used in Example #6 in the section on a two-stage BOD removal and nitrification process.

Solution - The solution is shown in the three figures on the next several pages, which are screenshots of different parts of an Excel worksheet used to carry out the calculations for this example. The first figure is from the top part of the worksheet and shows the user inputs and the calculation of the estimated **SARR/SALR** ratio for the

nitrification stage, resulting in a value of **SARR/SALR = 0.85.** The second figure is from the middle of the worksheet and shows the answers for parts i) through vi), as follows:

- i) The NO₃-N loading rate will be (1.5 MGD)(35 3.3 mg/L)(8.34 lb/MG/mg/L) = 396.6 lb/day = (396.6 lb/day)*(453.59 g/lb) = 179,879 g NO₃-N/day
- ii) Required carrier surface area = $(179,879 \text{ g/day})/(2 \text{ g/m}^2/\text{day}) = 89,939 \text{ m}^2$. Required carrier volume = $89,939 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = **149.90 m**³
- iii) For 40% carrier fill: Required tank volume = $149.90 \text{ m}^3/0.40 = 374.75 \text{ m}^3$.
- iv) The volume of liquid in the reactor can be calculated as: tank volume [carrier volume(1 void %)], Thus the volume of liquid is: $374.75 [149.90(1 0.60)] = 314.79 \text{ m}^3 = 314.79 *3.2808^3 = 11,117 \text{ ft}^3$.
- v) The HRT at design ave ww flow can be calculated as: $HRT_{des\ ave}$ = reactor liquid volume*7.48/[Q*10⁶/(24*60)] = 11,117*7.48/[1.5*10⁶/(24*60)] = **80 min**
 - $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 80/4 = 20 min$
- vi) Calculation of the estimated effluent NO₃-N concentration from the second stage, as shown above for the BOD removal process, is shown in the screenshot below and gives a value of <u>4.8 mg/L</u>,
- vii) Calculation of the alkalinity requirement is shown in the third spreadsheet screenshot below. Using the equivalent weight of CaCO₃ as 50, the equivalent weight of NaHCO₃ as 84, the alkalinity use for nitrification as 7.14 g CaCO₃/g NH₃-N, the alkalinity produced by denitrification as 3.56 g CaCO₃/g NO₃-N denitrified, and the target effluent alkalinity as 80 mg/L as CaCO₃, give the calculated alkalinity requirement as 81.9 mg/L as CaCO₃. The rate of alkalinity addition needed can then be calculated as: (1.5 MGD)(81.9 mg/L)*8.34 = 1024.9 lb/day as CaCO₃. Multiplying this by the ratio of the equivalent weight of NaHCO₃ to the equivalent weight of CaCO₃ gives the daily NaHCO₃ requirement as 1721.8 lb/day NaHCO₃.
- viii) Calculation of the methanol requirement in lb/day is shown at the bottom of the third screenshot below. As shown, the calculations use the constants, 4.6 lb COD/lb NO₃-N removed and 1.5 lb COD/lb Methanol. The required methanol dosage is then calculated as: 4.6/1.5 = 3.1 lb methanol /lb NO₃-N removed. The methanol requirement in lb/day is then equal to 3.1 times the previously calculated NO₃-N removal rate of 337.1 lb/day, or **1033.7 lb/day**.

Instructions: Enter value	s in blue	boxes. S	Spreadsh	eet calculate	es values i	n yellow b	oxes
I. Wastewater Parameter	Inputs						
1. Parameters for All Thre	a Stance						
1. I didilleters for All Tille	e stages						
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Prim. Effl. TKN Conc. =	35	mg/L	Pi	rim. Effl. NO ₃ -N	Conc. =	0	mg/L
Peak Hour Factor =	4	Pri	m. Effl. All	kalinity =	140	mg/L as C	aCO ₃
				55 25			
2. Parameters for First (BC	OD Remo	val) Stag	A CONTRACTOR OF THE PARTY OF TH			1	
			Da	ta points for SA		The second of the	26.5
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Area Loading Rate (SALR) =	. 6	g/m-/d		SARR/S/		0.925	0.875
See information on typical de values for SALR at right.	esign			alues above D removal vs			
Est. of SARR/SALR Rato =	0.935						
(Surf. Area Removal Rate/Sur	f. Area Loa	ding Rate)		Slope, SAF	RR/SALR vs	SALR:	-0.007
			Intercept, SARR/SALR vs SALR:				0.975
3. Parameters for Second	(Nitrifica)	tion) Stan					
o. Parameters for Second	HVILLINICA	HOII) Stay					
		PACONING CONTINUES	<u>. </u>				
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Influent NH ₄ -N Conc. = Min Design Temp., T =	25 45	mg/L °F	Target Eff	I NH ₄ -N Conc.			mg/L O Conc.
Charles where the history of the charles of the cha	7	mg/L	Target Eff		d on arrow	to select D.0	
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An and the section of	45	mg/L °F	Target Eff	n cell H26 and D.O Conc. i	d on arrow n Reactor =	to select D.0	O Conc.
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Tank L:W ratio =	1.5			(from carrier mfr/v	endor - c	only needed	to
(target L:W - only used if ta	nk is recta	ngular)		calculate hydrau	ılic deten	tion time)	
IV. Calculation of Carrie			quire	d Tank Volume 8	<u>Dimen</u>	sions	
3. Third Stage (Post-Anox	ic) Calcu	lations					m ³
3. Third Stage (Post-Anox Design Carrier Fill % =	ic) Calcu	(for third		Required Tank Volu	ume =	472.86	m³
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading =	ic) Calcu	(for third s			ume =		ft ³
3. Third Stage (Post-Anox Design Carrier Fill % =	32% 396.6	(for third		Required Tank Volu	ume =) =	472.86 16699.0	433
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading =	32% 396.6 179879	(for third solutions)		Required Tank Volume	ume = e = /idth =	472.86 16699.0 124909	ft ³ gal
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed =	32% 396.6 179879 89939	(for third solutions) (for third solution) (for third solution) (for third solution) (for third solution)	stage)	Required Tank Volume Tank Liquid Volume Calculated Tank W	ume = e = /idth = ength =	472.86 16699.0 124909 37.3 56.0	ft ³ gal ft
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = Estimate of NO ₃ -N Surface Ar	32% 396.6 179879 89939 149.90 14581.6	(for third sold) (for third sold) (b)day g/day m² m³	stage)	Required Tank Volume Tank Liquid Volume Calculated Tank W Calculated Tank Le	ume = e = /idth = ength = ition Time	472.86 16699.0 124909 37.3 56.0	ft ³ gal ft
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume =	32% 396.6 179879 89939 149.90 14581.6	(for third sold) (for third sold) (b)day g/day m² m³	stage)	Required Tank Volume Tank Liquid Volume Calculated Tank W Calculated Tank Le	ume = /idth = ength = tion Time	472.86 16699.0 124909 37.3 56.0	ft ³ gal ft
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = Estimate of NO ₃ -N Surface Ar	32% 396.6 179879 89939 149.90 14581.6	(for third solutions) (for third solution) (for thi	stage) Nor	Required Tank Volume Tank Liquid Volume Calculated Tank W Calculated Tank Leminal Hydraulic Reten	ume = /idth = ength = ntion Time Flow = ow =	472.86 16699.0 124909 37.3 56.0 at	ft ³ gal ft ft min
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = Estimate of NO ₃ -N Surface Ar Removal Rate, SARR =	32% 396.6 179879 89939 149.90 14581.6 ea	(for third so the solutions) (for third so the solution solution) (for third solut	stage) Nor	Required Tank Volume Tank Liquid Volume Calculated Tank W Calculated Tank Le minal Hydraulic Reten Design Average F	ume = /idth = ength = ntion Time Flow = ow = Conc.:	472.86 16699.0 124909 37.3 56.0 at 105 26 4.8	ft ³ gal ft ft min min
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = Estimate of NO ₃ -N Surface Ar Removal Rate, SARR =	32% 396.6 179879 89939 149.90 14581.6 ea 1.70 152897	(for third solutions)	stage) Nor	Required Tank Volume Tank Liquid Volume Calculated Tank W Calculated Tank Le minal Hydraulic Reten Design Average F Peak Hourly Flo	ume = /idth = ength = ntion Time Flow = ow = Conc.: Effl. NO ₃ -l	472.86 16699.0 124909 37.3 56.0 at 105 26 4.8	ft ³ gal ft ft min min
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = Estimate of NO ₃ -N Surface Ar Removal Rate, SARR = Est. of NO ₃ -N Removal Rate:	32% 396.6 179879 89939 149.90 14581.6 ea 1.70 152897	(for third solutions)	stage) Nor	Required Tank Volume Tank Liquid Volume Calculated Tank W Calculated Tank Le minal Hydraulic Reten Design Average F Peak Hourly Fle Calculated Effl NO ₃ -N	ume = /idth = ength = tion Time flow = conc.: Effl. NO ₃ -I sign valu	472.86 16699.0 124909 37.3 56.0 at 105 26 4.8 N conc. Is	ft ³ gal ft ft min min

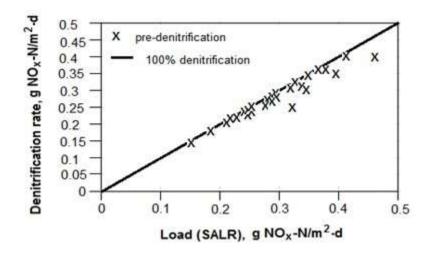
Screenshot – Post-Anoxic-Denitrification Design Calculations – Part 2

VII. Calcu	ulation of Alkali	nity Req	uirement	<u>s</u>				
Input:	Targ	et Effluent /	Alkalinity =		80	mg/L as CaC	O ₃	
Constant	s needed for Ca	culations	<u>:</u>					
Equiv W	t. of CaCO ₃ =	50	g/equiv.		Equiv Wt.	of NaHCO ₃ =	84	g/equiv.
Alkalin	ity used for Nitrifica	tion =	7.14	g Ca(CO3/g NH3-N re	emoved		
Alkalinity	produced by Denitr	ification =	3.57	g Ca(CO ₃ /g NO ₃ -N r	emoved		
Calculation	ons							
	Alkalinity to be ad	ded =	81.9	mg/L	as CaCO ₃			
Daily Alkalinity Requirement =		1024.9	lb/day as CaCO₃					
For sod	ium bicarbonate	use to add	alkalinity:					
Dail	y NaHCO₃ Requirer	nent =	1721.8	lb/da	y NaHCO ₃			
VII. Calcu	ulation of Carbo	on Source	e Require	emen	ts_			
Inputs:	Ca	rbon Sourc	e to be use	d:	Methanol	1		
	COD Requireme	nt for Denit	rification =		4.6	Ib COD/Ib NO:	3-N removed	
	COD Conter				1.5	lb COD/lb Car	bon Source	
Calculatio	ons							
	Carbon Source Do	sage =	3.1	Ib Ca	rbon Source/lb	NO ₃ -N removed		
Daily Ca	rbon Source Requi	rement =	1033.7	lb/da	у			

Screenshot – Post-Anoxic Denitrification Design Calculations – Part 3

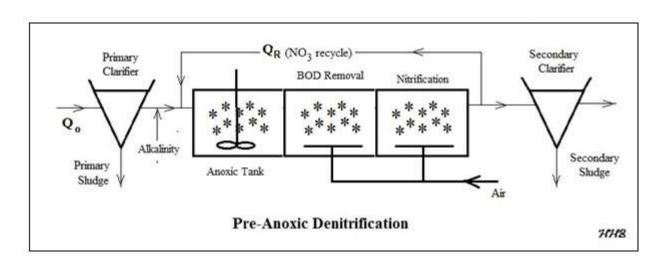
11. Pre-Anoxic Denitrification Process Design Calculations

The process design calculations for pre-anoxic denitrification, are similar to those just discussed for a post-anoxic denitrification process. The graph shown in the figure below (prepared using values from a similar graph in Rusten and Paulsrud's presentation in Ref #4 at the end of this course) will be used to obtain values for SARR/SALR vs SALR for the pre-anoxic denitrification stage.



SARR vs SALR for Pre-Anoxic Denitrification

A typical flow diagram for a pre-anoxic denitrification MBBR process is shown in the figure below. As discussed previously for a post-anoxic denitrification MBBR system, process design of a pre-anoxic denitrification MBBR system also requires sizing an MBBR tank for BOD removal, one for nitrification and one for denitrification. Process design for the nitrification stage is essentially the same as just discussed for the post-anoxic denitrification process.



MBBR Flow Diagram for Pre-Anoxic Denitrification

Process Design calculations for the BOD removal stage will be a bit different from those for the BOD removal stage in the post-anoxic process, because some of the incoming BOD is used as the carbon source for the denitrification reactions in the pre-anoxic tank. For the pre-anoxic process, the BOD loading rate (in lb/day) should be calculated as:

BOD Daily Loading = $(Q_0*S_0*8.34) - (0.67*(20/7)*NO_3-N \text{ removal rate})$

The second term is the estimated BOD removal rate in the anoxic reactor through its use in the denitrification reactions. This gives a lower BOD loading rate than that for the post-anoxic process with the same primary clarifier effluent coming in. Hence, the required tank size for BOD removal will be smaller for the pre-anoxic process.

The main difference from the post-anoxic denitrification process design calculations is for the denitrification stage, which will be discussed and illustrated with **Example #8** calculations below.

Example #8: Carry out the process design as described below for the denitrification stage of a pre-anoxic denitrification process with the wastewater flow and concentrations given in **Example #**7. [1.5 MGD containing 175 mg/L BOD and 35 mg/L TKN (in the primary effluent)]. Consider that the primary effluent alkalinity is 140 mg/L as CaCO₃ and the design SALR for the denitrification stage is to be 0.9 g NO₃-N/m²/d. The nitrification stage was designed for an effluent NH₃-N concentration of 3.3 mg/L, a D.O. of 3.0 mg/L, and a minimum WW temperature of 45°F.

For the first stage (denitrification) calculate each of the following:

- i) The nitrate loading rate
- ii) The required carrier volume for a carrier with specific surface area of 600 m²/m³
- iii) The required MBBR tank volume for a design carrier fill % of 40%
- iv) The volume of liquid in the MBBR reactor for design carrier % void space of 60%.
- v) The average hydraulic retention time at design average wastewater flow and at design peak hourly flow if the design peak hour factor is 4.
- vi) The required NO₃-N recycle rate needed to achieve a target effluent NO₃-N concentration of 9 mg/L.
- vii) The alkalinity requirement in lb/day as CaCO₃ and in lb/day NaHCO₃, for a target effluent alkalinity of 80 mg/L as CaCO₃.

Solution - The solution is shown in the three figures on the next several pages, which are screenshots of different parts of an Excel worksheet used to carry out the calculations for this example. The first figure below is from the top part of the worksheet and shows the user inputs and the calculation of the estimated **SARR/SALR** ratio for the denitrification stage (calculated to be **0.927**). Note also that a user input value is needed for the estimated NO₃-N recycle ratio. This initial estimated value is used in an iterative calculation farther down on the worksheet (in the second screenshot below) to

determine the required NO₃-N recycle ratio needed to achieve the target effluent NO₃-N concentration.

Instructions: Enter value	s in blue	boxes. S	preadsheet calculat	es values in	yellow box	es
I. Wastewater Parameter	Inputs					
1. Parameters for All Three	Stages					
Design ww Flow Rate, Q _o =	1.5	MGD	Prim. Effl. B	OD, S _o =	175	mg/L
Prim. Effl. TKN Conc. =	35	mg/L	Prim. Effl. NO	-N Conc. =	0	mg/L
Peak Hour Factor =	4	P	im. Effl. Alkalinity =	140	mg/L as CaCO ₃	
2. Parameters for First (Pre	-Anoxic	Stage:				
			Data points for			
Target Effl. NO ₃ -N Conc. =	9	mg/L	SALR	g/m ² /d):	0.2	0.5
Est. of NO ₃ -N Recycle Rato:	2.72		SARR	SALR:	0.95	0.94
(Q _R /Q _o) - An estimate is need	led here t	o start	(default values above	are from a	graph of Pre	Anoxic
the iterative calculation in S	ec IV belo	ow	SARR vs SALR in	ref #6 below	right)	
Design Value of NO ₃ -N Surface		(The graph is sh				
Area Loading Rate (SALR) =	0.9	g/m²/d	Slope, S	ARR/SALR vs	SALR:	-0.033
See information on typical de	sign		Intercept, S	ARR/SALR vs	SALR	0.957
values for SALR at right.			Est	of SARR/SAI	LR Rato =	0.927
			(Surf. Area Removal	Rate/Surf. Are	a Loading Rat	(e)

The figure, on the next page, is from the middle of the worksheet and shows the answers for parts i) through vi). The calculations and results are as follows:

- i) Most of the nitrate loading to the pre-anoxic denitrification tank is typically in the NO_3 -N recycle flow rather than in the primary effluent flow entering the tank. The NO_3 -N loading rate will be: (1.5 MGD)(Prim Effl NO_3 -N)(8.34 lb/MG/mg/L)
- + (1.5 MGD)(Recycle Ratio)(Target Effl NO₃-N)(8.34) = 306.5 lb/day
- = $(306.5 \text{ lb/day})*(453.59 \text{ g/lb}) = 139,003 \text{ g NO}_3-N/day$
- ii) Required carrier surface area = $(139,003 \text{ g/day})/(0.9 \text{ g/m}^2/\text{day}) = 154,447 \text{ m}^2$.

Required carrier volume = $154,447 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = 257.41 m^3 = $(257.41 \text{ m}^3)(3.2808^3 \text{ ft}^3/\text{m}^3)$ = 9090 ft^3 .

iii) For 40% carrier fill: Required tank volume = 9090 ft³/0.40 = <u>22,726 ft³</u>. (643.5 m³)

II. Determine whether to	use Pre-	Anoxic o	r Pos	t-Anoxic Denitrification			
Carbon:Nitrogen Ratio, C/N =	5.0		Use t	his Worksheet for Pre-And	oxic		
Target % N removal =	74%		Denit	trification calculations			
III. Carrier and Tank Sha	pe Paran	neter Inp	uts fo	r all Three Stages			
Carrier Spec. Surf. Area =	600	m ² /m ³	Click	on green box and then on			
(from carrier mfr/vendor)			arro	w to Select Tank Shape:	recta	ngular	
Liquid Depth in Tank =	8	ft		Carrier % Void Space =	60%		
Tank L:W ratio =	1.5			(from carrier mfr/vendor - o	only needed	to	
(target L:W - only used if tan	k is rectang	jular)		calculate hydraulic deten	tion time)		
IV. Calculation of Carrie	r Volume	and Rec	uired	Tank Volume & Dimens	ions		
1. First Stage (Pre-Anoxic	Tank) Cal	culations	•	(Carrier fill % is typically be			
				values are more conservati		and the second control and	and the second
Design Carrier Fill % =	40%	(for first s	tage)	expansion or reduction of S	- 150	370	rrier.
NO ₃ -N Daily Loading =	306.5	lb/day		Calculated Tank Volume =	643.5	m ³	
NO ₃ -N Daily Loading =	139002.7	g/day			22726.1	ft ³	
Carrier Surf. Area needed =	154447.4	m ²			169991	gal	
Calculated Carrier Volume =	257.412	m ³		Calculated Tank Width =	43.5	ft	
Tank Liquid Volume =	19089.9	ft ³		Calculated Tank Length =	65.3	ft	
			Nor	minal Hydraulic Retention Time	UKS0004985	VV02:00	
Estimate of NO ₃ -N Surface Ar	2000000	D		Design Average Flow =	137	min	
Removal Rate, SARR =	0.83	g/m²/d		Peak Hourly Flow =	34	min	
Req'd NO ₃ -N Removal Rate:	283.98	lb/day	Е	st. of NO ₃ -N Removal Rate:	283.98	g/day	
(for Effl. NO ₃ -N = Target Value	ue)		E	st. Rem Rate - Req'd Rate:	0.0000	g/day	
NOTE: Use Excel's "Goal	Seek" to fi	ind Q _R /Q _o	as follo	ows: Place the cursor on ce	ll H73 and o	click on	
"Goal Seek" (in the "tools"	menu of o	lder versi	ons an	d under "Data - What if Ana	lysis" in ne	wer	
versions of Excel). Enter v	alues to "S	et cell:"	H73, "	To value:" 0, "By changing	cell:" c17,	and	
				ear in cell C17 and cell H73			
Particular and the second seco				imate of Q _R /Q _o is needed in			
the iterative process.							

iv) The volume of liquid in the reactor can be calculated as:

tank volume – [carrier volume(1 – void %)]. Thus, the volume of liquid is:
$$22,726 - [9090(1 - 0.60)] = 19,090 \text{ ft}^3$$
. (540.6 m³)

v) The HRT at design ave WW flow can be calculated as:

HRT_{des ave} = reactor liquid volume*7.48/[Q*10⁶/(24*60)] = $19,090*7.48/[1.5*10^{6}/(24*60)] = 137 \text{ min}$

 $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 137/4 = 34 min$

- vi) The required NO₃-N recycle ratio is calculated with the iterative process described in blue at the bottom of the last figure above. For this iterative process, the NO₃-N removal rate is calculated two different ways, one using the estimated SARR and the carrier surface area while the other uses the wastewater flow rate times the influent TKN concentration minus the sum of the effluent nitrate and ammonia nitrogen concentrations. Excel's Goal Seek process is then used to set the difference between the two different calculations equal to zero by changing the estimated value of the NO₃-N recycle ratio. This process results in the required NO₃-N recycle ratio calculated to be **2.72**.
- vii) Calculation of the alkalinity requirement is shown in the third screenshot figure, below on this page. Using the equivalent weight of $CaCO_3$ as 50, the equivalent weight of $NaHCO_3$ as 84, the alkalinity use for nitrification as 7.14 g $CaCO_3/g$ NH_3 -N, the alkalinity produced by denitrification as 3.56 g $CaCO_3/g$ NO_3 -H, and the target effluent alkalinity as 80 mg/L as $CaCO_3$, give the calculated alkalinity requirement as **97.1 mg/L** as $CaCO_3$. The rate of alkalinity addition needed can then be calculated as: (1.5 MGD)(91.1 mg/L)*8.34 = 1214.5 Ib/day as $CaCO_3$. Multiplying this by the ratio of the equivalent weight of $NaHCO_3$ to the equivalent weight of $CaCO_3$ gives the daily $NaHCO_3$ requirement as 12040.3 Ib/day 120.3 NaHCO3.

VII. Calculation of Al						
nput:	Target Effluent	Alkalinity =	80	mg/L		
Constants needed for	Calculations:					
Equiv Wt. of CaCO ₃ =	50	g/equiv.	Equiv Wt. o	of NaHCO ₃ =	84	g/equiv.
Alkalinity used for Nit	rification =	7.14	g CaCO ₃ /g NH ₃ -N rer	noved		
Alkalinity produced by D	enitrification =	3.57	g CaCO ₃ /g NO ₃ -N rer	moved		
Calculations						
Alkalinity to b	e added =	97.1	mg/L as CaCO ₃			
Daily Alkalinity Re	quirement =	1214.5	Ib/day as CaCO ₃			
For sodium bicarbon	ate use to add a	alkalinity:				
Daily NaHCO₃ Red	uirement =	2040.3	Ib/day NaHCO₃			

12. Summary

The MBBR (moving bed biofilm reactor) process is an attached growth process that uses plastic carriers to provide a surface on which biofilm grows. The plastic carriers are kept suspended in the aeration tank by an aerator for an aerobic process or by mechanical mixing for an anoxic or anaerobic process. The plastic carriers are kept in the system by a sieve at the outlet of the tank. The MBBR process doesn't require sludge recycle, because the biomass remains in the system attached to the plastic carriers. The required reactor size for an MBBR process is typically significantly smaller than that for an activated sludge process treating the same wastewater flow, or for other common attached growth processes like the RBC or trickling filter. It can be used for BOD removal, biological nitrification, biological denitrification, and biological phosphorus removal.

This course provides discussion of and detailed examples of process design calculations for a single stage BOD removal MBBR system, a two-stage BOD removal MBBR system, a single stage tertiary nitrification MBBR system, a two-stage BOD removal/Nitrification MBBR system a post-anoxic denitrification MBBR system and a pre-anoxic denitrification MBBR system.

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